

Atrium Medical Corporation

[Headquarters Facility]

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→ Title Page



Pennsylvania State University

Architectural Engineering
Senior Thesis Final Presentation



Ref: Atrium Medical Project Documents

Jeffrey Martin
Advisor: Dr. Leicht
April 16th, 2014

Atrium Medical Corporation

Headquarters Facility



Ref: Atrium Medical Project Documents

40 Continental Boulevard
Merrimack NH, 03054

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- Owner Information
- Building Information



Project Information:

Location: 40 Continental Boulevard,
Merrimack, NH 03054

Site Size: 2,367,100 SF

Existing Structure: 2 Story building
114,000 SF

Previous Owner: Fidelity Investments

New Owner: Atrium Medical
Corporation/ Maquet
Getinge Group

Project Scope: Existing Renovation
101,200 SF New Addition

Existing Conditions

Proposed 101,200 SF New Addition (Footprint)



Ref: www.google.com/maps

Existing 100,000SF Building to be Renovated

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Project Information:

Project Owner: Atrium Medical Corporation/
MAQUET/GETINGE Group

Previous Owner: Fidelity Investments

Reason for Purchase: Company Expansion
Bring all 450 + Employees
Into One Facility.

Divisions of Work: Manufacturing, Storage,
Business Offices, R&D,
Engineering Shops



Ref: www.classiccapital.net

Owner Information



Ref: www.theiddoctor.info

- Specializes in R&D and Manufacturing
 - Cardiology
 - Radiology
 - Chest Trauma
 - Thoracic Drainage
- Business unit of MAQUET Cardiovascular (Structured Alliance)
- Member of GETINGE Group of companies

Jeffrey Martin | Advisor: Dr. Robert Leicht | Final Presentation

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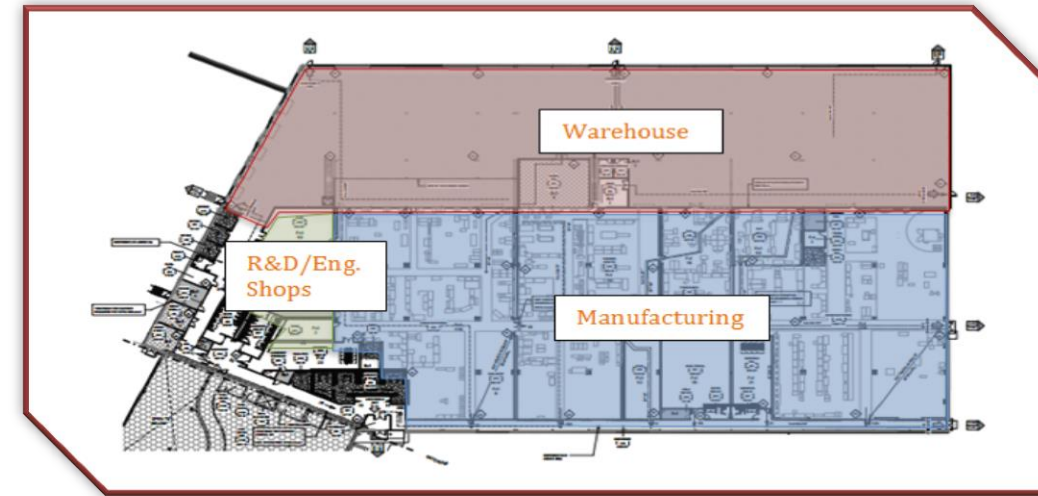
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Project Information

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- Owner Information
- Building Information



Project Information:



Ref: Atrium Medical Project Documents

- Project Location: 40 Continental Boulevard, Merrimack, NH 03054
- Building Size: 101,200 SF
- Zoning: I-3 Industrial
- Description: Single Story Building
Interior Mezzanine

Building Information

- CM Firm:
- Architect:
- Structural Engineer:
- Civil Engineer:
- Mechanical Engineer:
- Electrical Engineer:

Hutter Construction
Lavallee Brensinger
Foley Buhl Roberts
Hayner Swanson Inc.
Johnson & Jordan Inc.
Gate City Electric



Ref: All Images found on www.google.com/images

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Depth Analysis 1:

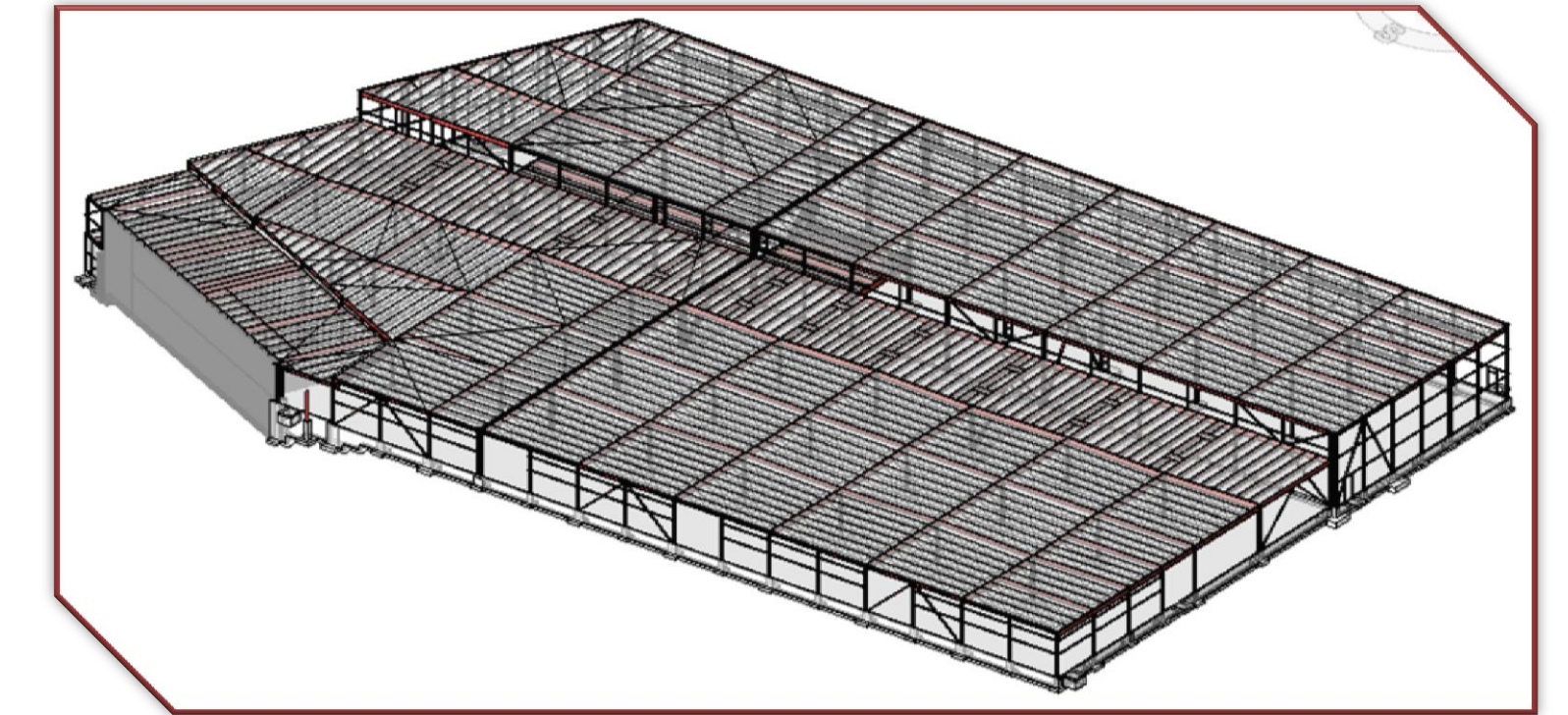
Description of Structure:

- Structure: Steel Superstructure
- Beams: Wide Flange Steel Beams
- Columns: Wide Flange Steel Columns
- Roof Joists: K-Series Joists
- Lateral Bracing: HSS Steel Sections
- Foundations: Concrete spread & strip footings and piers

Problem: Owner not utilizing the opportunity to develop a more efficient structure, in regards to either cost or scheduling.

Problem Statement

Steel Structure: Atrium Medical Corporation



Ref: Atrium Medical Revit Structural Model

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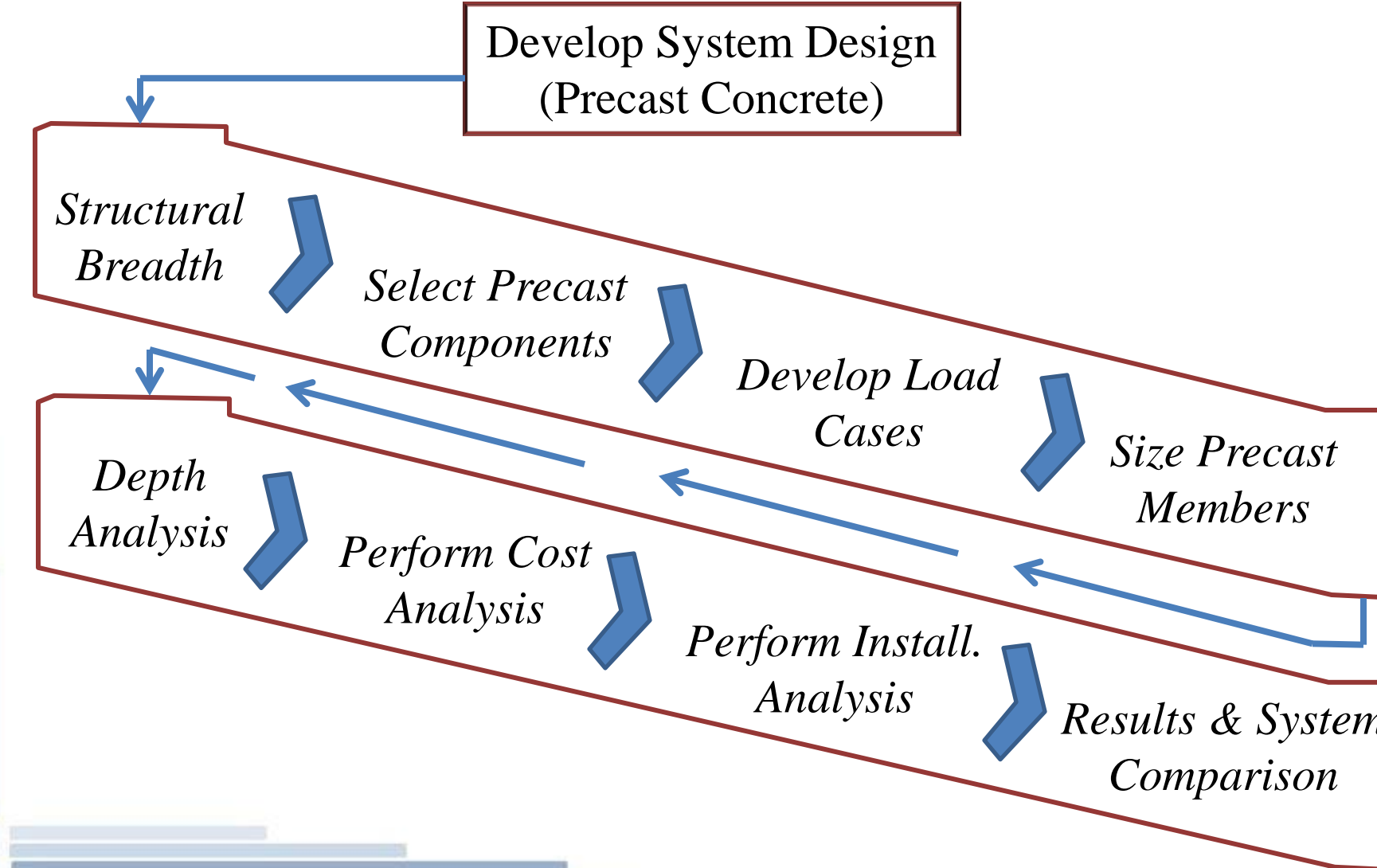
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Depth Analysis 1:

Sequence of Events:



Proposed Solution

Advantages of Precast Structures:

- Decrease in Project Schedule
- Saves Space on-site
- Saves Money (labor)



Disadvantages of Precast Structures:

- Availability
- Timing
- Small Margin of Error
- High Material Cost



Ref: All Images found on www.google.com/images

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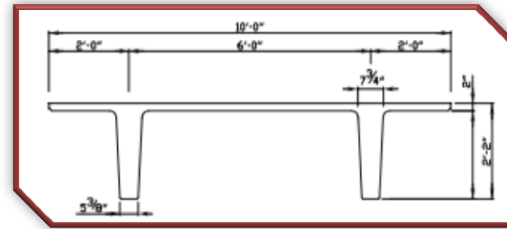
Depth Analysis 1

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- **Structural Breadth**
 - Developing a Design
 - Determining Loads
 - Sizing Members
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Depth Analysis 1:

Double Tee Beam

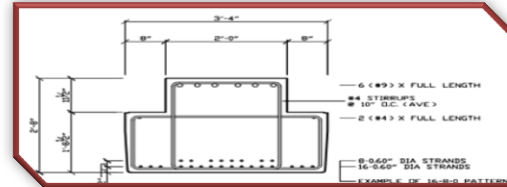


Ref: www.nitterhouse.com/technical-info



Ref: www.concretetech.com

Inverted Tee Beam

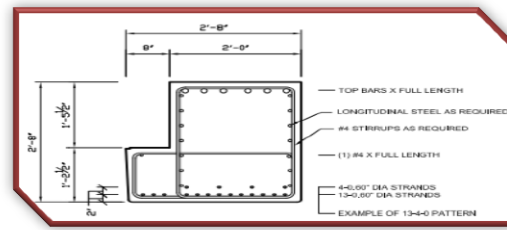


Ref: www.nitterhouse.com/technical-info



Ref: www.dynaspan.com

Ledger Beam



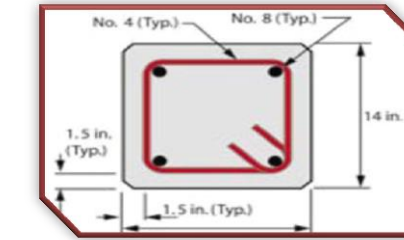
Ref: www.nitterhouse.com/technical-info



Ref: www.cpm-group.com

Structural Breadth:

Square Concrete Columns

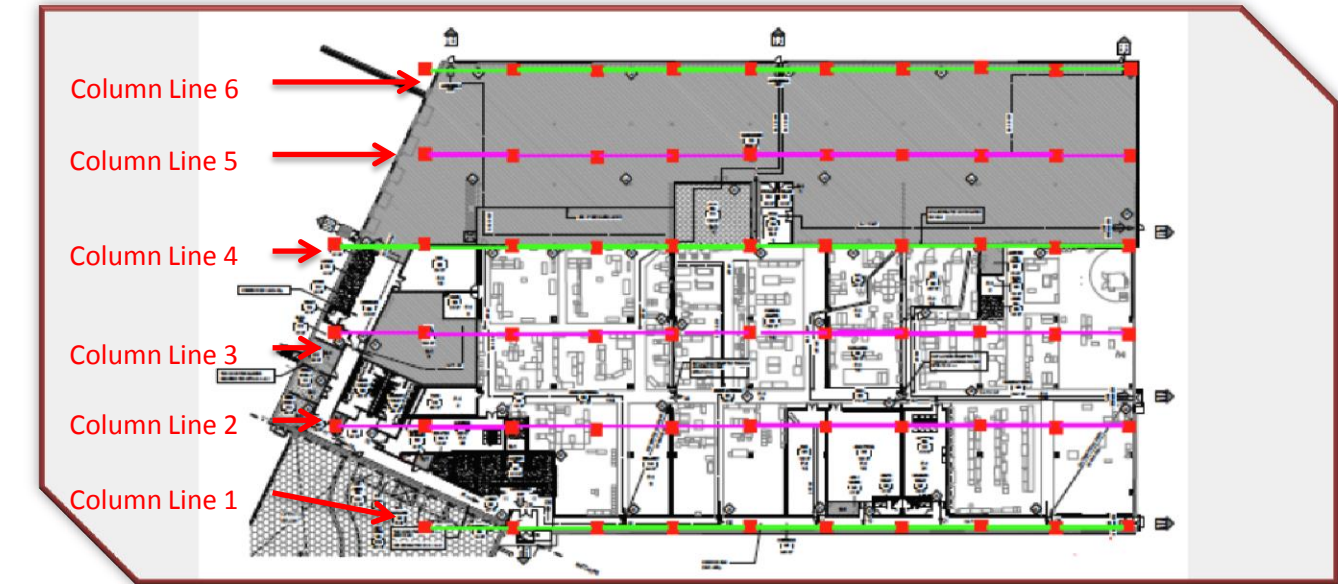


Ref: www.condor-rebar.com



Ref: www.timesunion.com

Proposed System Layout



Ref: Atrium Medical Project Documents

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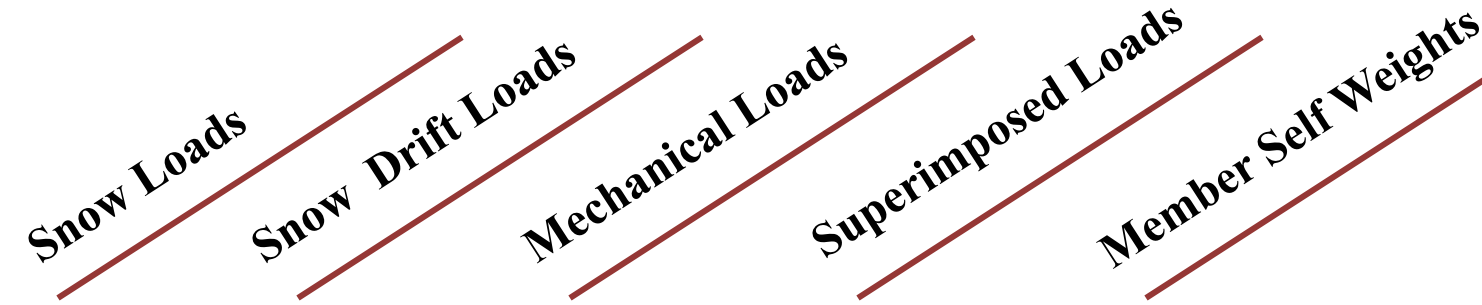
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Depth Analysis 1:



Snow Loads:

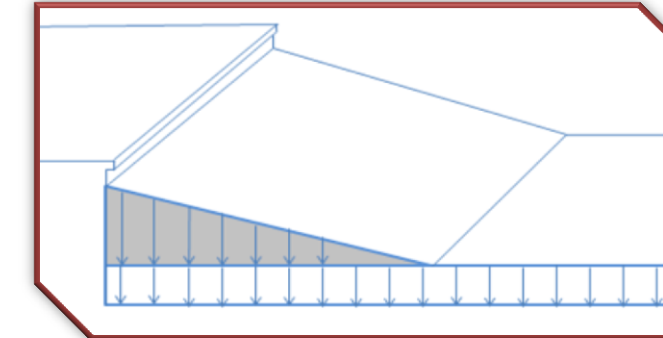
SNOW LOADS	
A.	GROUND SNOW LOAD - (ERDC/CRREL TR-02-0)
B.	FLAT ROOF SNOW LOAD - (ASCE 7-05 - SECTION 7.3)
C.	SNOW EXPOSURE FACTOR - (ASCE 7-05 - TABLE 7-2)
D.	SNOW IMPORTANCE FACTOR - (ASCE 7-05 - TABLE 7-4)
E.	ROOF THERMAL FACTOR - (ASCE 7-05 - TABLE 7-3)
F.	ROOF SLOPE FACTOR - (ASCE 7-05 - FIGURE 7-2)
G.	SNOW DRIFT - PER ASCE 7-05 - FIGURES 7-7, 7-8 & 7-9

$S(f) = 60 \text{ PSF}$
 $C(f) = 1.0$
 $P(f) = 42 \text{ PSF}$
 $C(e) = 1.0$

Ref: Atrium Medical Project Documents

- Snow loads from structural drawings
- Flat Roof Snow Load = 42 PSF

Snow Drift Loads:



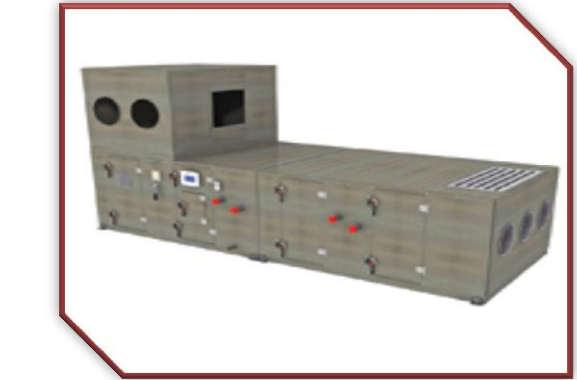
- Two main roof levels
- Lower Roof Height = 17' 0"
- Higher Roof Height = 27' 8"
- Max Surcharge = 92.31 PSF
- Surcharge Length = 16.94

Structural Breadth:

Mechanical Loads

- Loads due to (8) AHU's and (4) RTU's
- Act as point load(s) throughout roof
- Maximum AHU load = 9000 lbs.

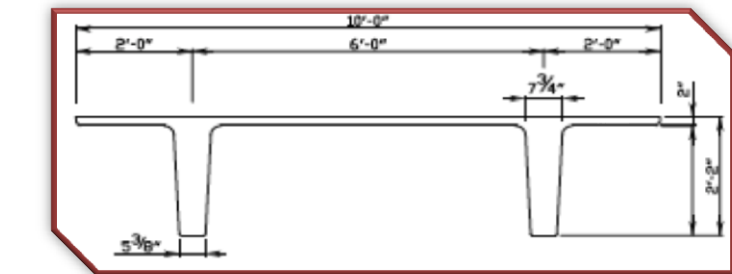
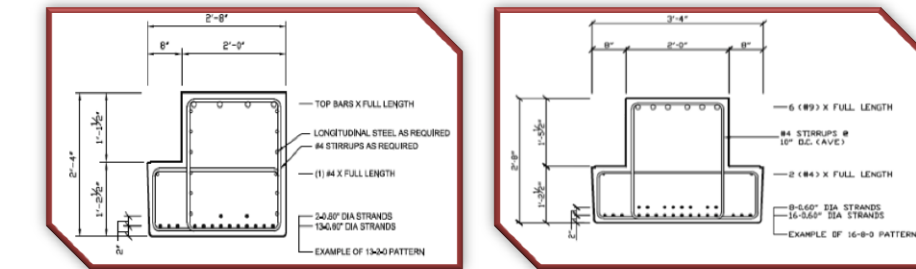
Determining Loads



Ref: www.trane.com

Member Self Weights & Superimposed Loads

- Loads from:
 - Double Tees
 - Ledger & Inverted Tee Beams
 - Superimposed Dead = 15 PSF



Ref: www.nitterhouse.com/technical-info

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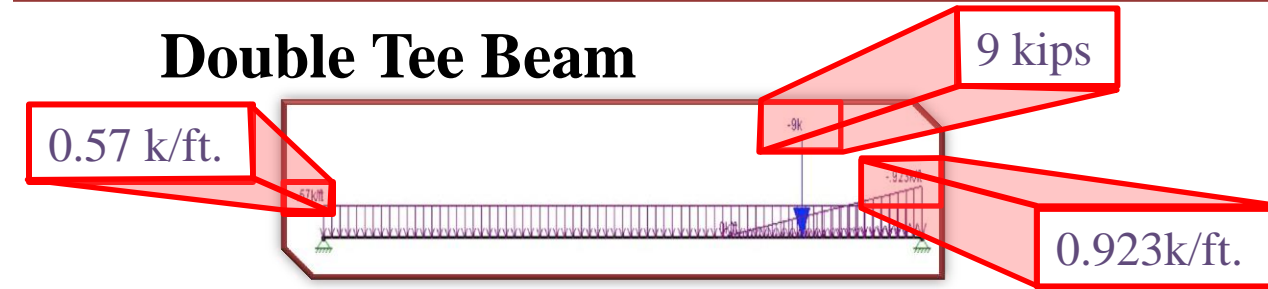
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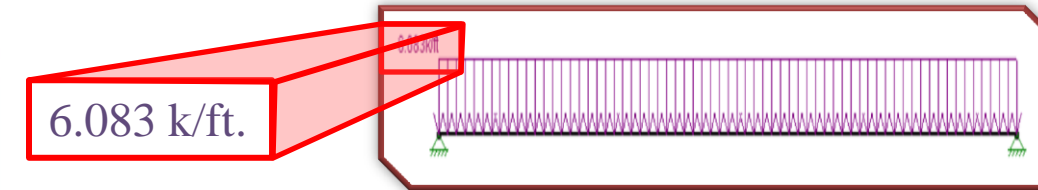
Double Tee Beam



Beam Designs

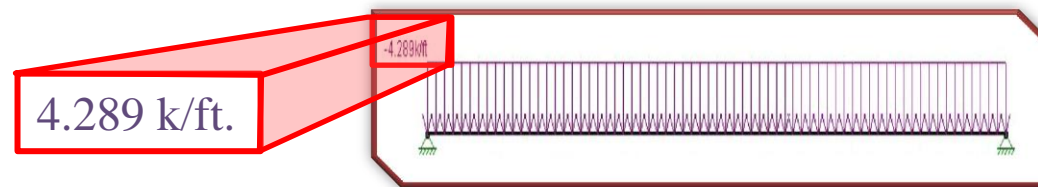
26"x10' D.T (No Topping), 26-6.6P

Inverted Tee Beam



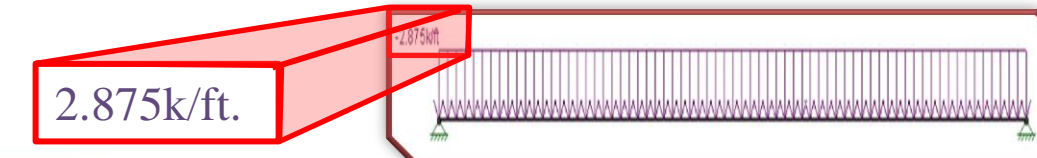
Inverted Tee Beam
40IT36-A

Interior Ledger Beam



32LB28 (SP 13-6-0)
(TB 6 - #9)

Exterior Ledger Beam



18LB32 (SP 6-4-0)
(TB 4 - #9)

Structural Breadth:

Sizing Members

Concrete Column Loads

Column Line #	Total Axial Loads
Column Line 1	178.4 kips
Column Line 2	409.6 kips
Column Line 3	426.8 kips
Column Line 4	463.1 kips
Column Line 5	409.6 kips
Column Line 6	178.4 kips

Concrete Column Designs

Designs Selected For Columns by Column Line	
Column Line 1	10" x 10" w/ 4 - #5 bars at 17 ft height
Column Line 2	10" x 10" w/ 4 - #10 bars at 17 ft height
Column Line 3	10" x 10" w/ 4 - #11 bars at 17 ft height
Column Line 4	12" x 12" w/ 4 - #8 bars at 27.5 ft height
Column Line 5	10" x 10" w/ 4 - #10 bars at 27.5 ft height
Column Line 6	10" x 10" w/ 4 - #5 bars at 27.5 ft height

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 - Cost Summary
 - Installation Summary
- Analysis Results



Depth Analysis 1:

Total Precast Cost Summary:

Type	Quantity	Length	Unit	Mat'l Cost/Unit	Total Mat'l Cost	Labor/Equip. Cost/Unit	Total Labor/Equip. Cost	
Double Tee Beam	188	50	LF	\$18.00	\$169,200.00	\$700.00	\$131,600.00	
Inverted Tee Beam	29	40	LF	\$275.00	\$319,000.00	\$700.00	\$20,300.00	
Interior Ledger Beam	10	40	LF	\$275.00	\$110,000.00	\$700.00	\$7,000.00	
Exterior Ledger Beam	28	40	LF	\$275.00	\$308,000.00	\$700.00	\$19,600.00	
Column Line 1	10	17	LF	\$275.00	\$46,750.00	\$700.00	\$7,000.00	
Column Line 2	11	17	LF	\$275.00	\$51,425.00	\$700.00	\$7,700.00	
Column Line 3	11	17	LF	\$275.00	\$51,425.00	\$700.00	\$7,700.00	
Column Line 4	11	27.5	LF	\$275.00	\$83,187.00	\$700.00	\$7,700.00	
Column Line 5	10	27.5	LF	\$275.00	\$75,625.00	\$700.00	\$7,000.00	
Column Line 6	10	27.5	LF	\$275.00	\$75,625.00	\$700.00	\$7,000.00	
				Total	\$1,290,237.00	Total	\$222,600.00	
							Total Initial System Cost	\$1,512,837.00

Total Design Summary:

Additional Footing Cost:

Footing Type	Original Cost	Cost Increase (35%)
Spread Footings	\$69,225.81	\$24,229.03
Strip Footings	\$25,675.92	\$8,986.57
Additional Concrete Cost		\$33,216.00

Initial System Cost: \$1,512,837.00
 +
 Additional Footing Cost: \$33,216.00
 =
Total System Cost: \$1,546,053.00

Cost Summary



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Depth Analysis 1:

Total Precast Installation Summary:

Type	Quantity
Double Tee Beam	188
Inverted Tee Beam	29
Interior Ledger Beam	10
Exterior Ledger Beam	28
Column Line 1	10
Column Line 2	11
Column Line 3	11
Column Line 4	11
Column Line 5	10
Column Line 6	10
Total Members	318
# Picks per day	~ 6 to 8
Days for completion	40 to 53 Days

Total Design Summary:



Ref: www.timesunion.com



Ref: www.dynaspan.com

Installation Summary



Ref: www.concretetech.com

Total System Installation Time:

40 to 53 days

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Depth Analysis 1:

Overall Systems Comparison and Analysis Results:

	Total Cost	Installation Time (days)
Precast Structural System (Proposed System)	\$1,546,053.00	53 to 40
Steel Structural System (Original System)	\$1,273,160.00	45
Difference	(+) \$272,893.00	(+) 8 to (-) 5

Issues:

- Cost is too high
- Schedule decrease not significant

Solution:

- Add another crane on-site

Analysis Results

Adjusted Costs and Installation Times:

Costs	
100 Ton Crane Rental Cost	\$18,000.00
Add'l Cost of Precast System	\$272,893.00
Total System Cost	\$290,893.00
Installation Times	
Total Steel Member Qty.	318
# Picks per day (one crane)	~6 to 8
# Picks per day (two cranes)	~12 to 16
Total System Installation Time (days)	20 to 26.5

Revised Precast System Cost: **\$1,564,053**

Revised Precast System Installation Time: **20 to 26.5 days**

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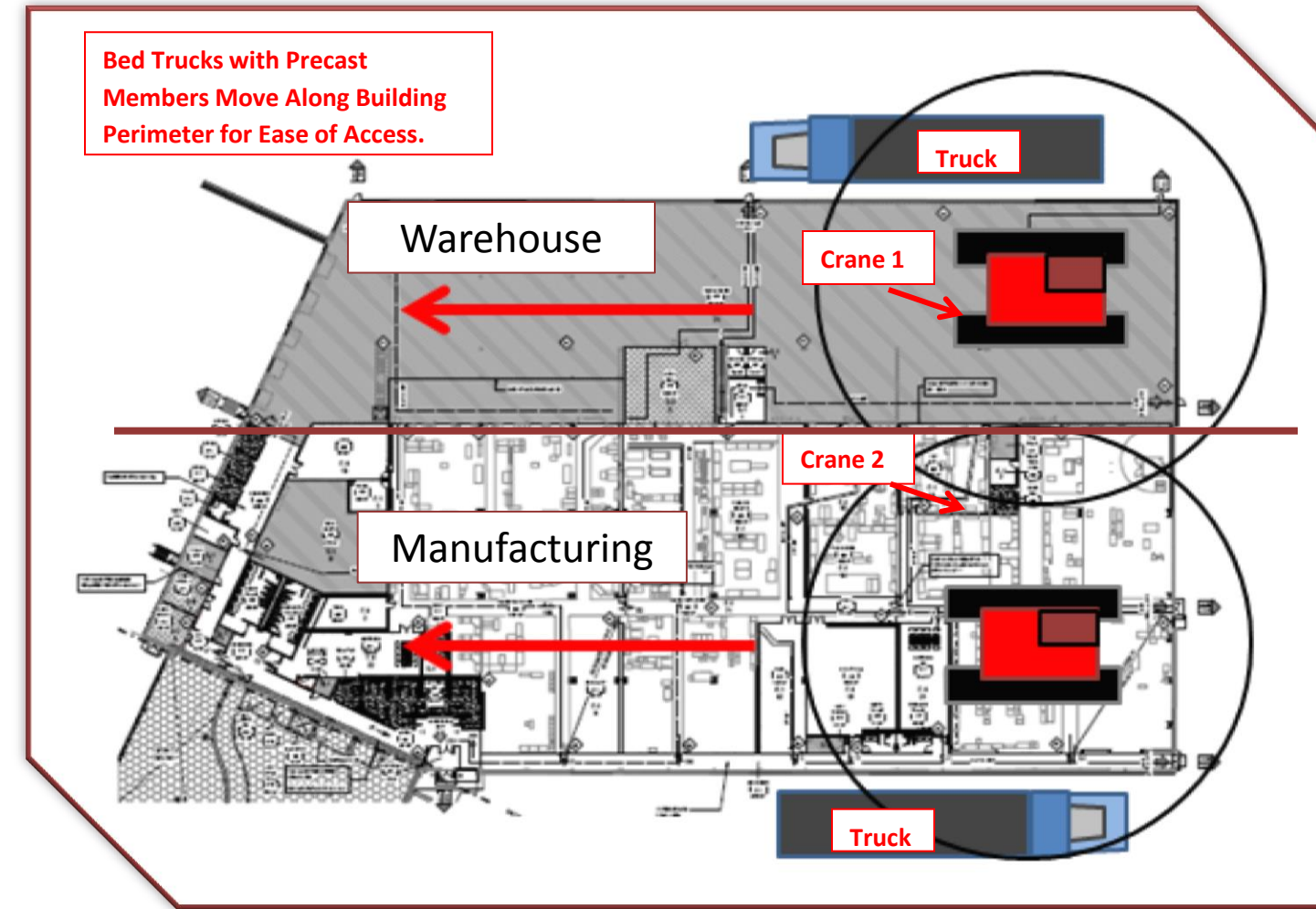
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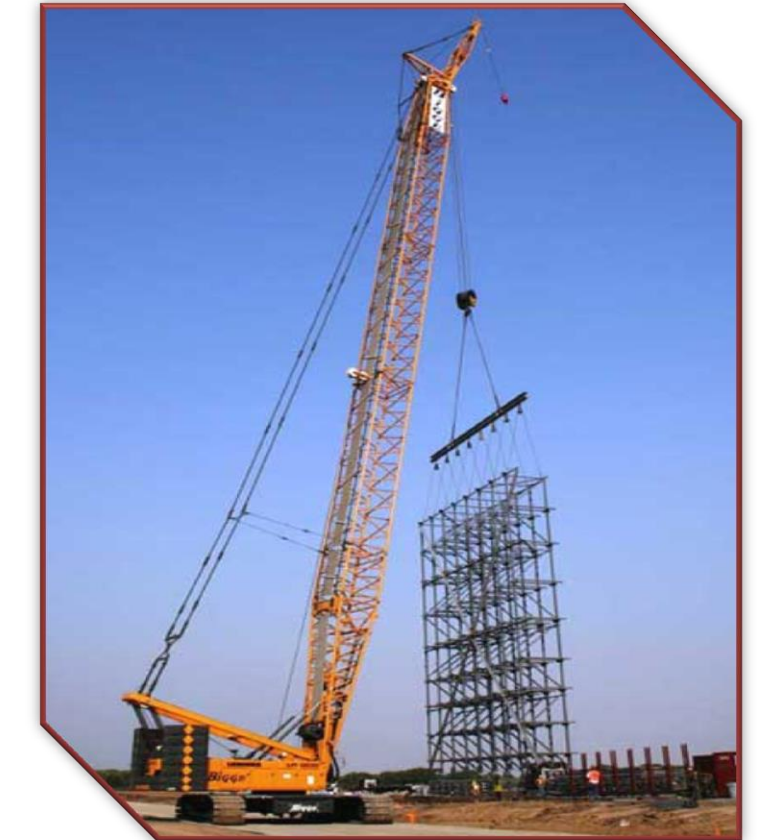
Depth Analysis 1:



Ref: Atrium Medical Project Documents

Analysis Results

- Project will utilize (2) 100 Ton Crawler Cranes
- Cranes will move within building footprint
 - (1) crane in Manufacturing area
 - (1) crane in Warehouse Area
- Movement of work flow from East to West
- Gives Total Installation time = 20 to 26.5 days



Ref: www.bigge.com



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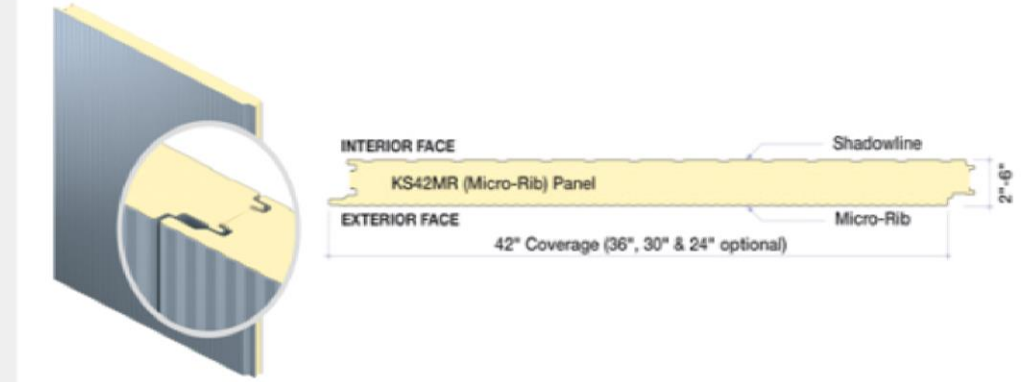
Depth Analysis 2:

Description of Envelope:

- Design: Kingspan Micro-Rib Insulated Metal Panels
- Location: Exterior Warehouse Area
- Area (SF)
 - Southern Face: 3,106 SF
 - Eastern Face: 2,788 SF
 - Northern Face: 10,401 SF
 - Western Face: 4,016 SF

Problem: Owner not utilizing the opportunity to create a more efficient building envelope surrounding the warehouse area.

Problem Statement



Product Specification

Panel Thickness ²	2-1/2" 3" 4" 5" 6"
R-Value	7.5 per inch
Panel Width	24" 30" 36" 42" (standard)
Lengths	8'-0" to 52'-0"
Joint Configuration	Double tongue and groove interlocking rainscreen joint
Reveals	Standard 1/8" vertical application, standard 3/8" horizontal application
Exterior Face	24 or 22 Ga. Micro-Rib profiled embossed G-90 galvanized or Galvalume® pre-painted steel
Interior Face	26 Ga. Shadowline profiled embossed G-90 galvanized or Galvalume® pre-painted steel
Orientation	Horizontal or Vertical
Product Code	KS42MR

Ref: www.kingspanpanels.us

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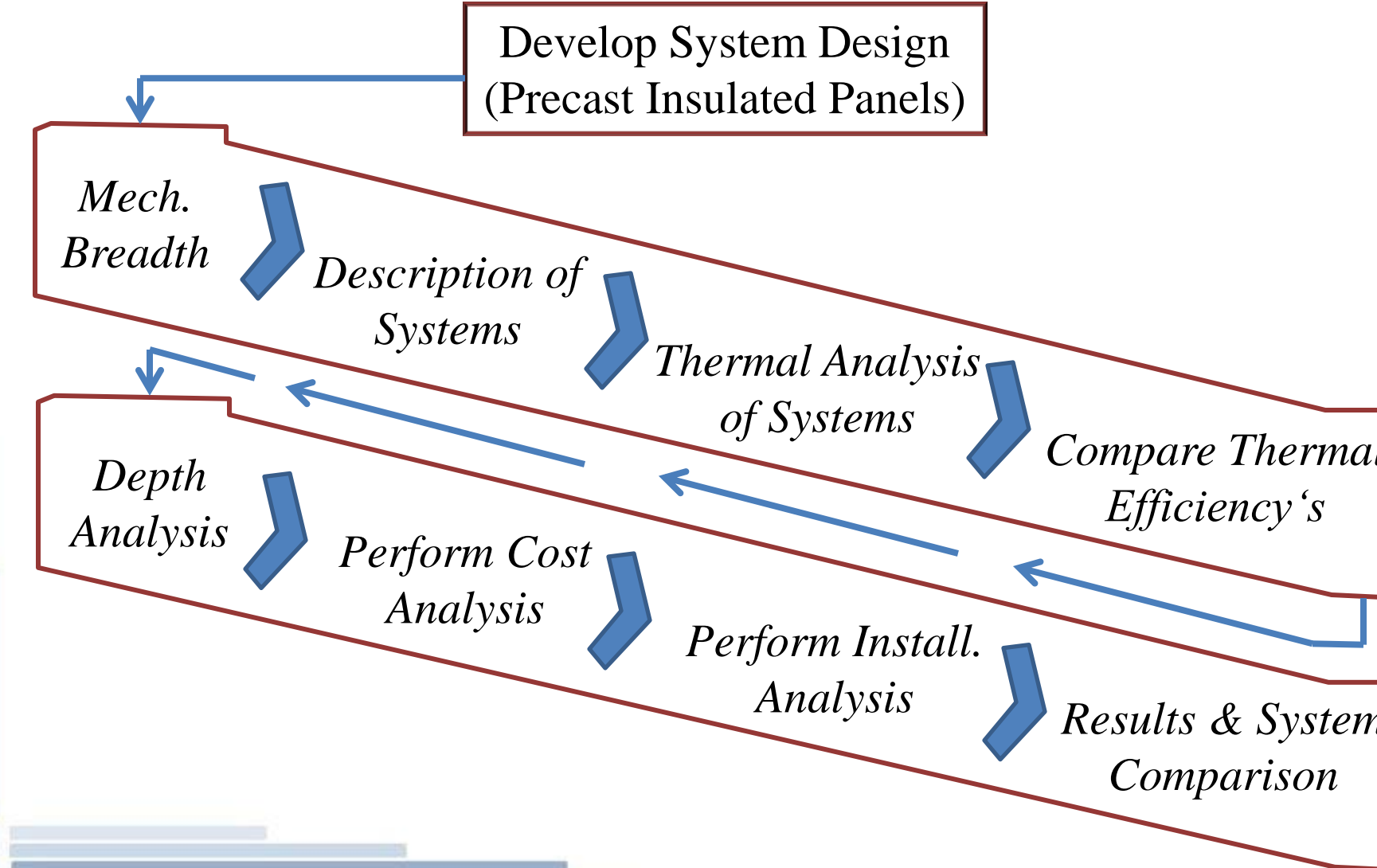
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Depth Analysis 2:

Sequence of Events:



Proposed Solution

Advantages of Precast Insulated Panels:

- Decrease Time in Project Schedule
- Versatility
- Energy & Thermal Efficiency
- Fire Resistance



Disadvantages of Precast Insulated Panels:

- High Materials Cost
- Timing



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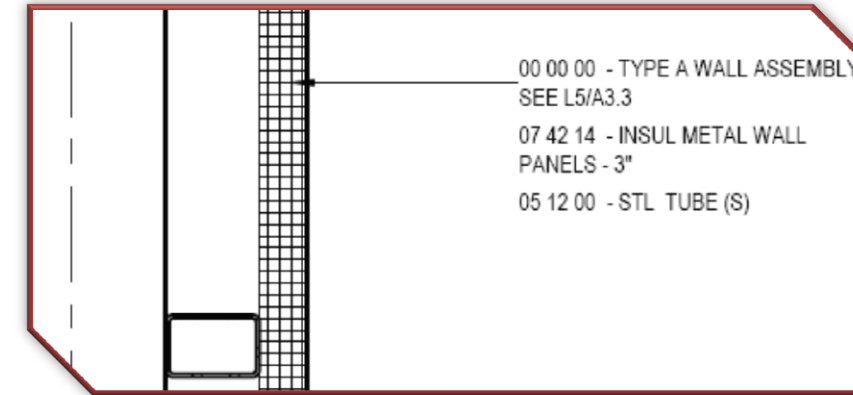
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- **Mechanical Breadth**
 - Description of Systems
 - Thermal Analysis & Results
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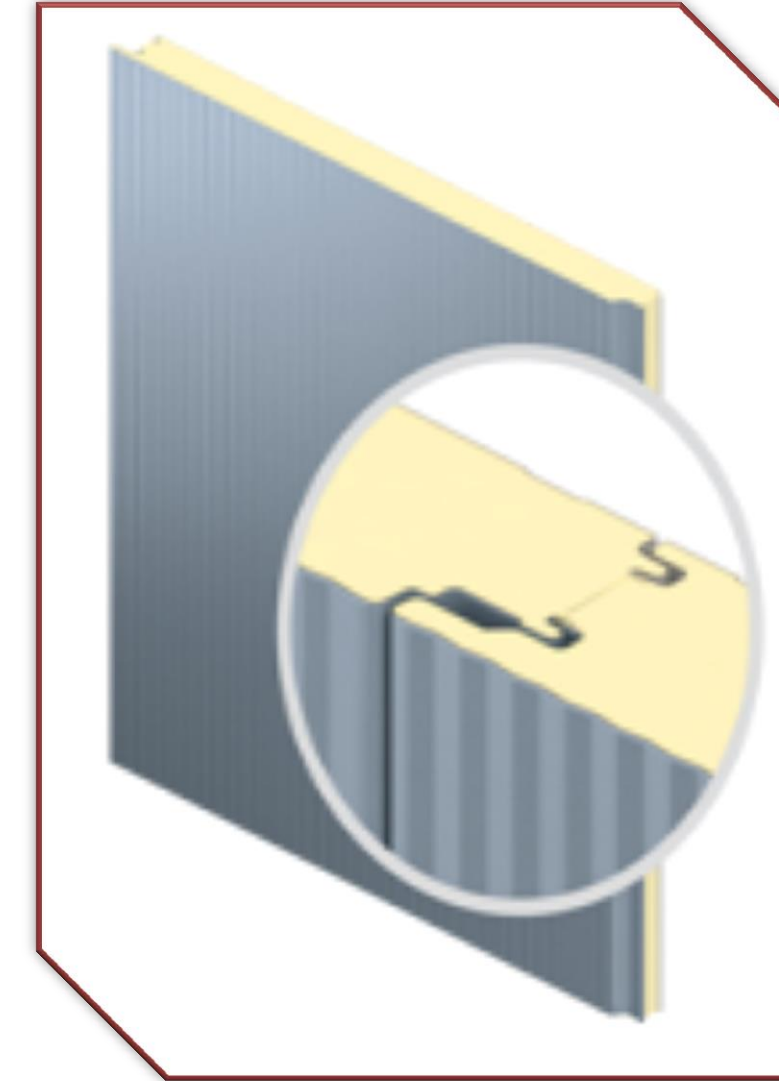


Depth Analysis 2:

Original System: Insulated Metal Panels



Ref: Atrium Medical Project Documents



Ref: www.kingspanpanels.us

Mechanical Breadth:

Proposed System: Precast Insulated Panels



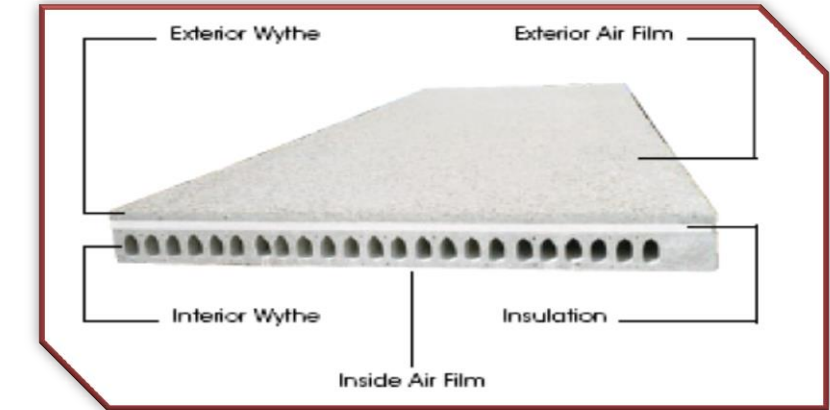
Ref: www.spancrete.com



Description of Systems



Ref: www.spancrete.com



Ref: www.spancrete.com



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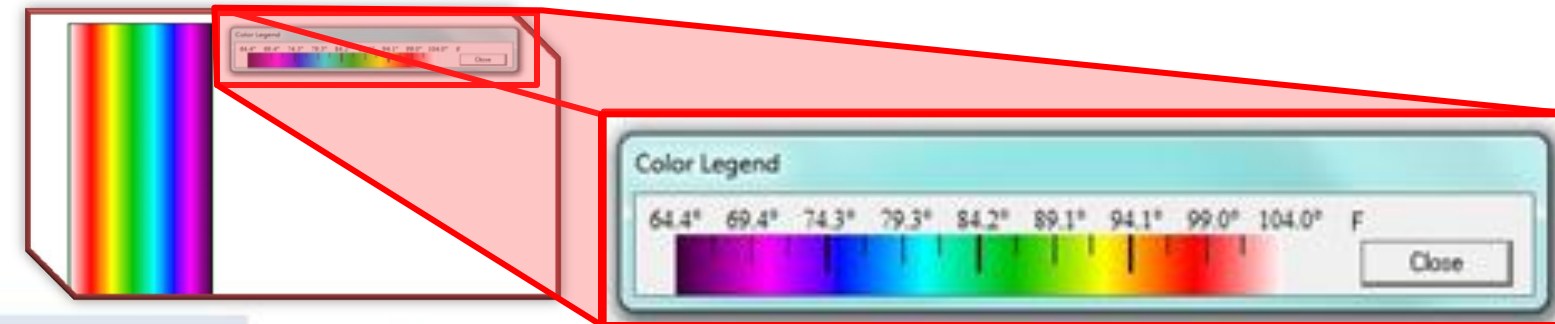
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Depth Analysis 2:

Insulated Metal Panels: Summer Conditions

Insulated Metal Panels						
Thermal Analysis: Heat Transfer (Extreme Summer Cond. Int = 64.4°F, Ext = 104°F)						
Outside (Ta)(°C) =	40	Inside (Td)(°C) =	18	$\Delta T_i = U * (T_a - T_d) * R_i$		
	Conductivity (k)	Thickness (m)	Conductance (Ci)	Resistance (Ri)	ΔT	T (°C)
Interior Temp.						
Int. Film	N.A.	N.A.	8.3	0.120481928	0.6797791	18.00
Metal Panel	18	0.00045466	39,590.02	2.52589E-05	0.0001425	18.68
Insulation	0.02	0.074985	0.27	3.74925	21.153894	39.83
Metal Panel	18	0.00075946	23,701.05	4.21922E-05	0.0002381	39.83
Ext. Film	N.A.	N.A.	34	0.029411765	0.1659461	40.00
				RSI Total =	3.899	
				R-Value =	22.140	
				U-Value =	0.256	



Mechanical Breadth: Thermal Analysis & Results

Insulated Metal Panels: Winter Conditions

Insulated Metal Panels						
Thermal Analysis: Heat Transfer (Extreme Winter Cond. Int = 64.4°F, Ext = -29°F)						
Outside (Ta)(°C) =	-34	Inside (Td)(°C) =	18	$\Delta T_i = U * (T_a - T_d) * R_i$		
	Conductivity (k)	Thickness (m)	Conductance (Ci)	Resistance (Ri)	ΔT	T (°C)
Interior Temp.						
Int. Film	N.A.	N.A.	8.3	0.120481928	-1.60675	18.00
Metal Panel	18	0.00045466	39,590.02	2.52589E-05	-0.00034	16.39
Insulation	0.02	0.074985	0.27	3.74925	-50.0001	-33.61
Metal Panel	18	0.00075946	23,701.05	4.21922E-05	-0.00056	-33.61
Ext. Film	N.A.	N.A.	34	0.029411765	-0.39224	-34.00
				RSI Total =	3.899	
				R-Value =	22.140	
				U-Value =	0.256	

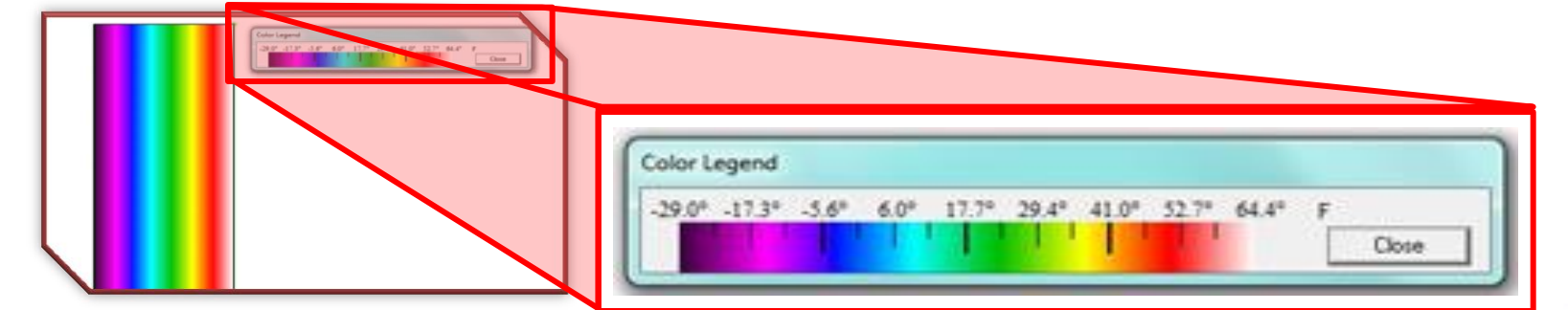




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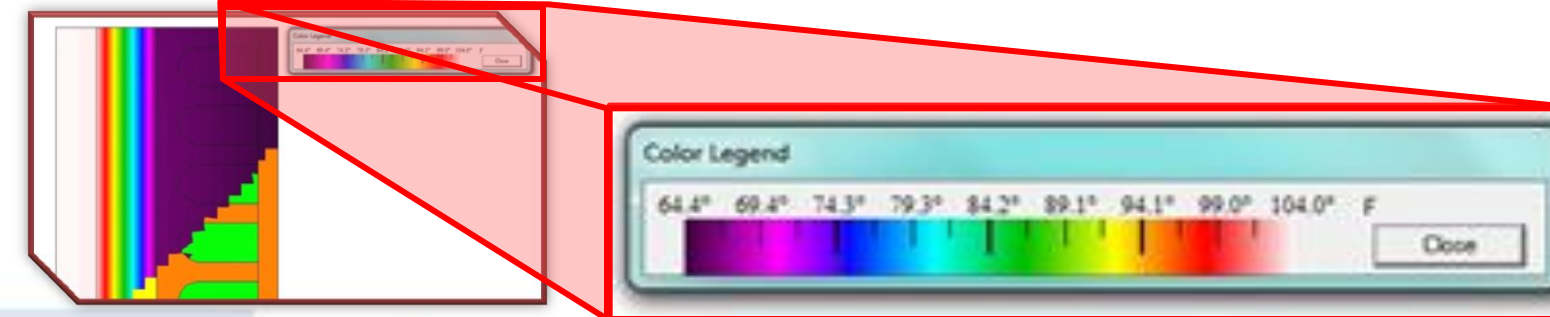
- Problem Statement
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Depth Analysis 2:

Precast Insulated Panels: Summer Conditions

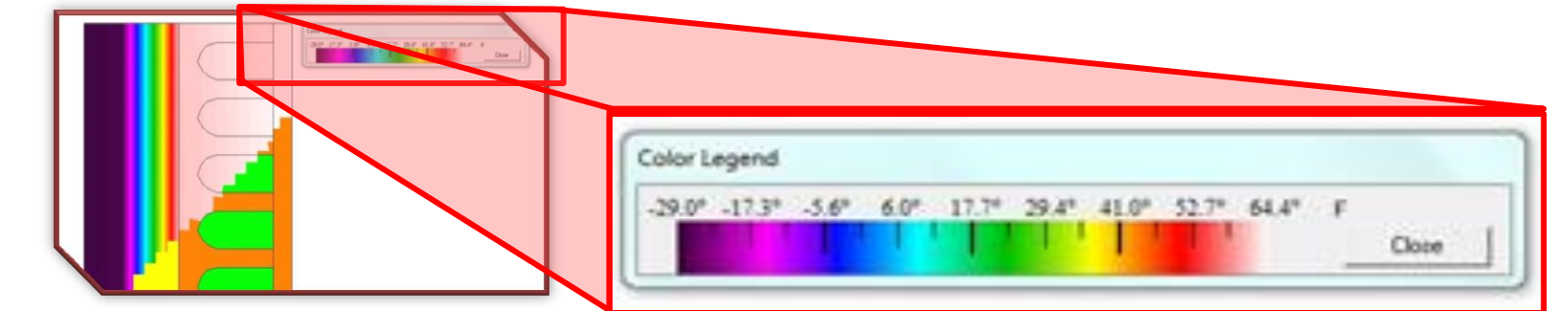
Precast Insulated Panels						
Thermal Analysis: Heat Transfer (Extreme Summer Cond. Int = 64.4°F, Ext = 104°F)						
Outside (Ta)(°C) =	40	Inside (Td)(°C) =	18	$\Delta T_i = U * (T_a - T_d) * R_i$		
	Conductivity (k)	Thickness (m)	Conductance (Ci)	Resistance (Ri)	ΔT	T (°C)
Interior Temp.						18.00
Int. Film	N.A.	N.A.	8.3	0.120481928	0.6326882	18.63
Concrete	0.7	0.1524	4.59	0.217714286	1.1432856	19.78
Insulation	0.02	0.074985	0.27	3.74925	19.688481	39.46
Concrete	0.7	0.0508	13.78	0.072571429	0.3810952	39.85
Ext. Film	N.A.	N.A.	34	0.029411765	0.1544503	40.00
				RSI Total =	4.189	
				R-Value =	23.788	
				U-Value =	0.239	



Mechanical Breadth: Thermal Analysis & Results

Precast Insulated Panels: Winter Conditions

Precast Insulated Panels						
Thermal Analysis: Heat Transfer (Extreme Winter Cond. Int = 64.4°F, Ext = -29°F)						
Outside (Ta)(°C) =	-34	Inside (Td)(°C) =	18	$\Delta T_i = U * (T_a - T_d) * R_i$		
	Conductivity (k)	Thickness (m)	Conductance (Ci)	Resistance (Ri)	ΔT	T (°C)
Interior Temp.						18.00
Int. Film	N.A.	N.A.	8.3	0.120481928	-1.49544	16.50
Concrete	0.7	0.1524	4.59	0.217714286	-2.70231	13.80
Insulation	0.02	0.074985	0.27	3.74925	-46.5364	-32.73
Concrete	0.7	0.0508	13.78	0.072571429	-0.90077	-33.63
Ext. Film	N.A.	N.A.	34	0.029411765	-0.36506	-34.00
				RSI Total =	4.189	
				R-Value =	23.788	
				U-Value =	0.239	



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Depth Analysis 2

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- Mechanical Breadth
- **Total Design Summary**
 - Cost Summary
 - Installation Summary
- Analysis Results



Depth Analysis 2:

Precast Insulated Panel Cost:

Location	Area (ft ²)	Unit	Material \$/Unit	Material \$
Southern Face	3106	SF	18	\$ 55,908.00
Eastern Face	2788	SF	18	\$ 50,184.00
Northern Face	10401	SF	18	\$ 187,218.00
Western Face	4016	SF	18	\$ 72,288.00
				\$ 365,598.00
Location	Quantity	Unit	Labor/Equip \$/Unit	Labor/Equip \$
Southern Face	14	Ea.	700	\$ 9,800.00
Eastern Face	13	Ea.	700	\$ 9,100.00
Northern Face	47	Ea.	700	\$ 32,900.00
Western Face	18	Ea.	700	\$ 12,600.00
				\$ 64,400.00
			Total Cost	\$ 429,998.00

Total Design Summary:

Cost Summary

Additional Footing Cost:

Footing Type	Original Cost	Cost Increase (35%)
Spread Footings	\$40,631.00	\$14,221.00
Additional Concrete Cost		\$14,221.00

Initial System Cost:	\$429,998.00
	+
Additional Footing Cost:	\$14,221.00
	=
Total System Cost:	<u>\$444,219.00</u>

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Depth Analysis 2:

Precast Insulated Panel Install Time:

Location	Area (ft ²)	Member Area (ft ²)	Quantity (Area/Member Area)
Southern Face	3106	221.36	14
Eastern Face	2788	221.36	13
Northern Face	10401	221.36	47
Western Face	4016	221.36	18
Total Quantity			92
# Picks per Day			~6 to 8
Total Installation Time			12 to 15

Total Design Summary:

Installation Summary



Ref: www.spancrete.com



Ref: www.spancrete.com

Total System Installation Time: 12 to 15 days

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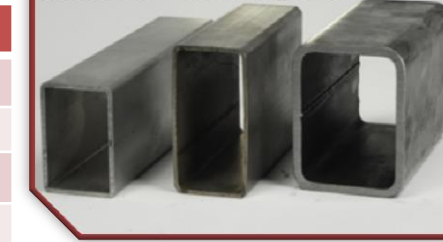


Depth Analysis 2:

Insulated Metal Panel Costs:

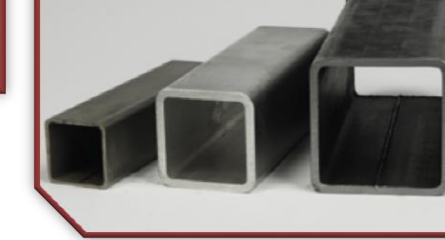
Insulated Metal Panel System Cost	
Subtotal	\$354,400.00
HSS Framing Cost (+)	\$46,355.00
Metal Panel Cost (-)	\$31,007.00
Total System Cost	\$369,748.00

Hollow Structural Sections -- Rectangular



Ref: www.bossteel.com

Hollow Structural Sections -- Square



Ref: www.bossteel.com

Insulated Metal Panel Installation Time:

Total Wall Panel System Installation Time =		67 Days	
System Type	Area (ft^2)	% of Install. Time	Total Install. Time (days)
Metal Wall Panels	7,112	26%	17
Insulated Metal Panels	20,311	74%	50
Total =	27,423	100%	67

Analysis Results

Overall Systems Comparison and Analysis Results:

	Total Cost	Installation Time (days)
Precast Insulated Panels	\$444,219.00	12 to 15
Insulated Metal Panels	\$369,748.00	50
Difference	(+) \$74,471	(-) 38 to (-) 35

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Depth Analysis 3:



Ref: www.hutterconstruction.com

Summary of Safety Plan:

- Formal safety plan for field and office staff *during* construction
- Superintendents have OSHA 30-Hour training
- All other employees have OSHA 10-Hour training.
- Weekly toolbox talks
- Basic construction safety (i.e. PPE, Equip. Safety etc.)

Problem: Owner not utilizing the opportunity to plan and design for safety consideration prior to project's construction.

Problem Statement

Hutter Construction on *Safety*:

“Training is an integral part of Hutter’s safety commitment. In addition to the traditional weekly toolbox talks, ongoing safety training is regularly provided by outside professionals. Among the topics continually addressed: competent persons, confined space, boom lifts, forklifts, snorkel lifts, CPR and first aid. All of our employees, including project managers have received the OSHA 10-Hour certified training and new employees receive the training within 30 days of hire. All of our superintendents have received OSHA 30-Hour training.”

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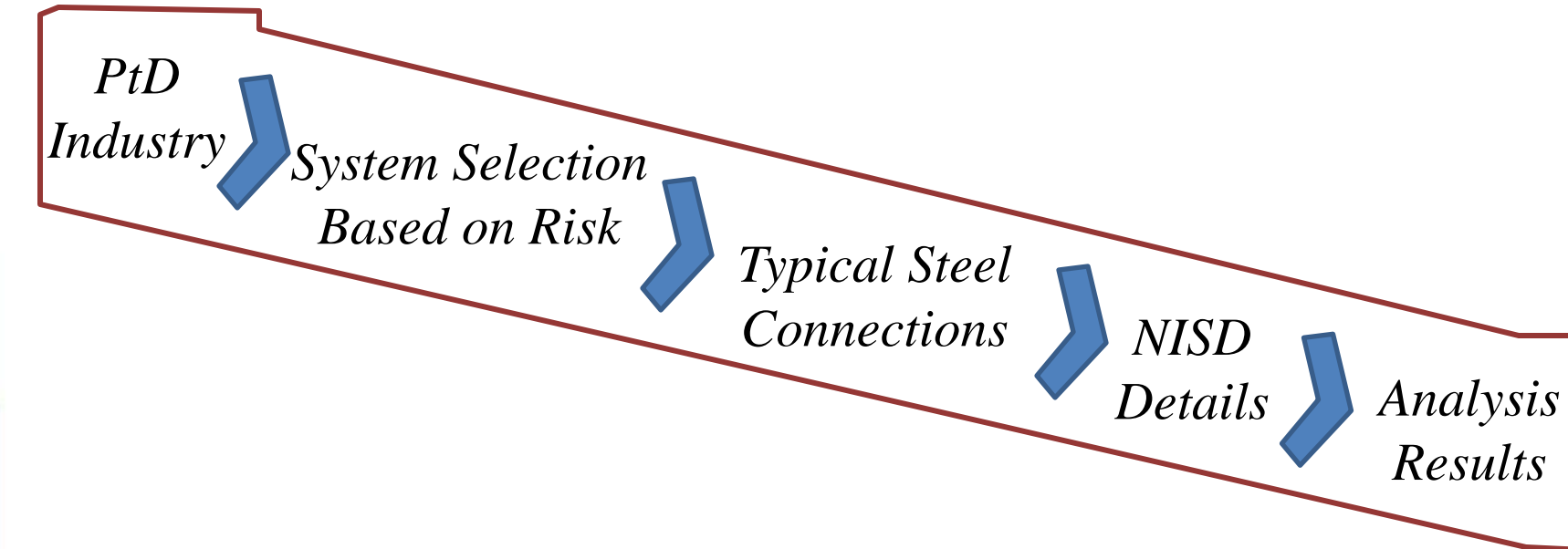
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Depth Analysis 3:

Sequence of Events:



Proposed Solution

Benefits of Implementing a Design Guide:

- Increase Safety Consideration
- Increase in Quality Control
- Reduce Delays
- Increase in Productivity
- Increase Collaboration between Designer & Constructors



Barriers of Implementing a Design Guide:

- Designer's Liability
- Additional Costs
- Lack of Expertise



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Depth Analysis 3:

Prevention through Design (PtD):

- Industry developed to prevent hazards from occurring during construction
 - Began in late 90's
 - Construction tasks & processes viewed during conceptual and design phase
 - Ensures safety of workers during construction



Ref: www.asse.org



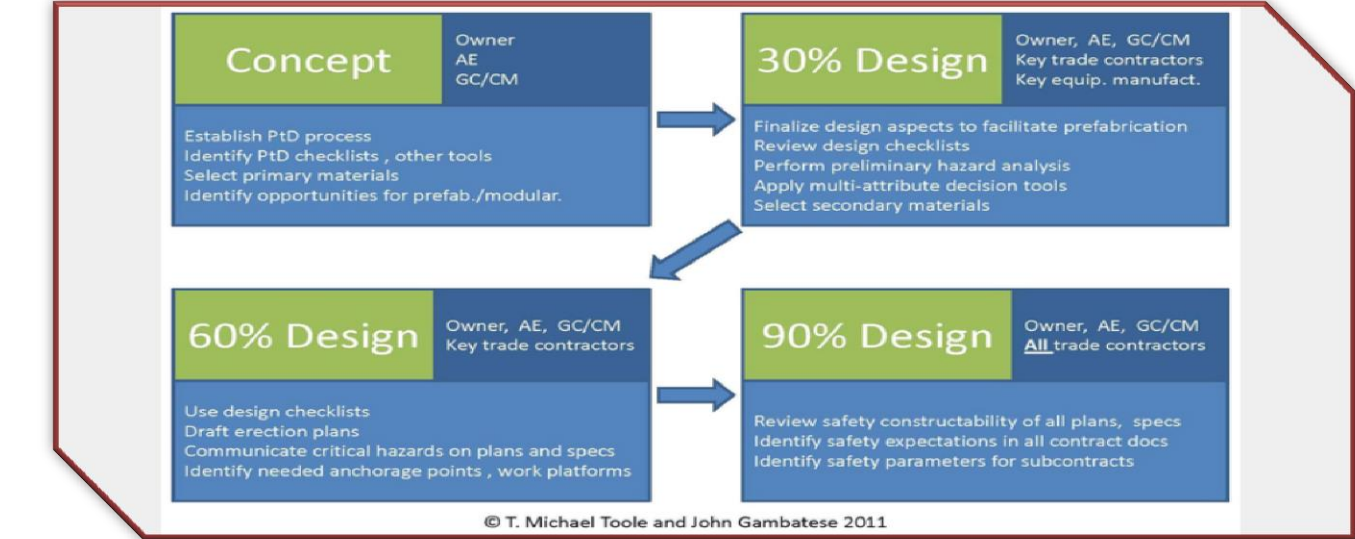
Ref: www.lhsfna.org



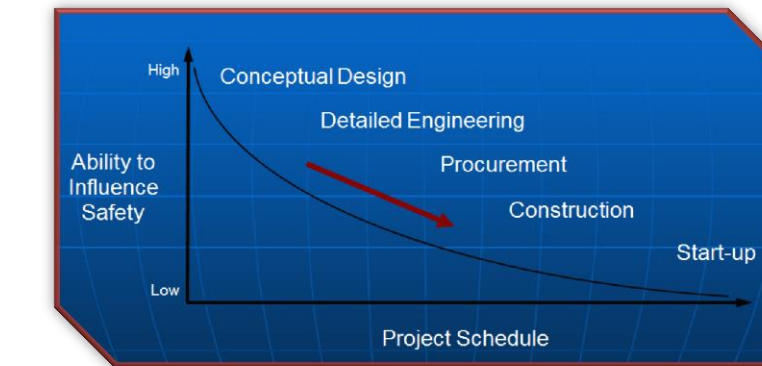
Ref: www.asse.org

Prevention through Design Industry

Prevention through Design Process:



Ref: www.designforconstructionsafety.org



Ref: www.elcosh.org

As the timeline of the project schedule increase, the ability to influence safety on the project decreases.

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Depth Analysis 3:

SLiDeRule Information:

- Safety in Design Risk Evaluator
- Program designed to interpret the level of construction safety risk for a particular project.
- This program is used primarily for:
 - Determining the level of safety risk for an entire building and each system within that building
 - Comparing designs based on risks
 - Learning about design features that could potentially increase or decrease risk
 - Creating building designs that minimize the risk of injury for construction workers

System Selection

SLiDeRule Results

System Name	Safety Risk	Risk Percentage
Foundation		4%
Structural Frame		28%
Exterior Enclosure		18%
Roof		15%
Interiors		5%
Plumbing		1%
HVAC		17%
Electrical		13%



Ref: www.constructionsliderule.org

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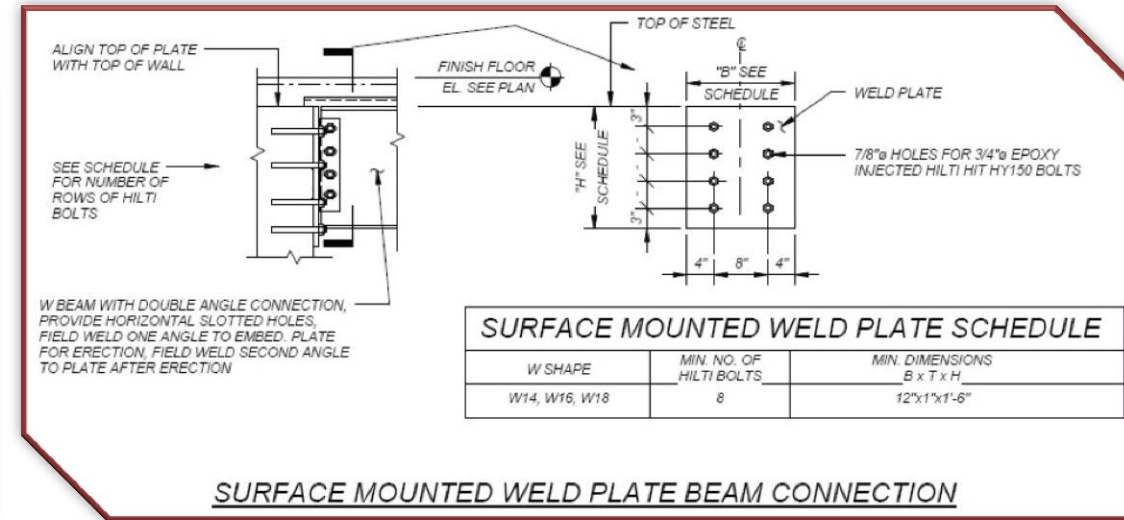
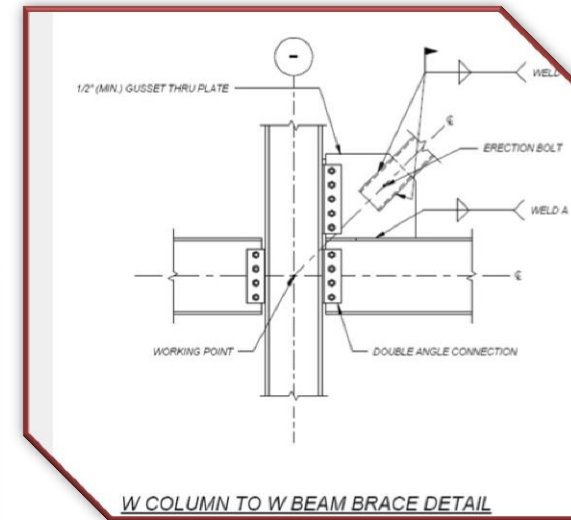
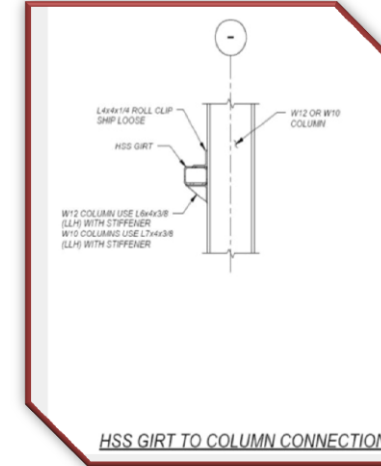
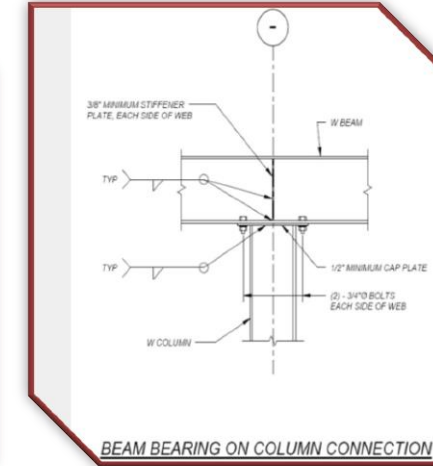
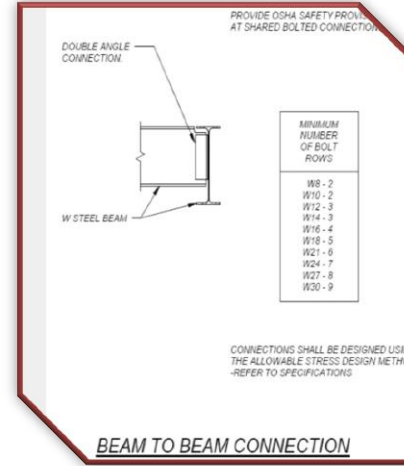
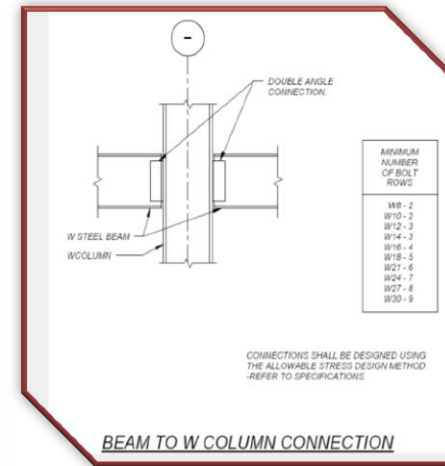
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Depth Analysis 3:



Typical Steel Connections

STRUCTURAL STEEL FRAMING

- S1. STRUCTURAL STEEL WORK SHALL CONFORM TO THE AISC "SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS - 360-05" AND AISC "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES - 2005", AS MODIFIED BY THE SPECIFICATIONS.
- S2. WELDING SHALL BE IN ACCORDANCE WITH AWS "D1.1 2006-STRUCTURAL WELDING CODE-STEEL".
- S3. STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING, UNLESS NOTED:

A. PLATES	ASTM A36	$F_y = 36\text{KSI}$
B. STRUCTURAL TUBING	ASTM A500 GRADE B	$F_y = 46\text{KSI}$ (SQUARE & RECTANGULAR TUBING), $F_y = 42\text{KSI}$ (ROUND TUBING)
C. ALL OTHER SHAPES	ASTM A992 OR A588 GRADE B	$F_y = 50\text{KSI}$
- S4. CONNECTIONS MAY BE BOLTED OR WELDED, UNLESS SPECIFICALLY NOTED OTHERWISE. CONNECTIONS SHALL BE DESIGNED AND DETAILED IN ACCORDANCE WITH AISC STANDARDS, USING THE ASD METHOD.
- S5. CONNECTIONS SHALL BE WELDED TO CONFORM TO ASTM A233, E70 SERIES, OR BOLTED TO CONFORM TO ASTM A325, TYPE N BOLTS.
- S6. PROVIDE 3/4" DIAMETER MINIMUM HEADED TYPE ANCHOR RODS AT COLUMNS AND POSTS, UNLESS NOTED OTHERWISE.
- S7. FURNISH AND INSTALL ONE WASHER AND ONE HEAVY HEX NUT WITH ALL ANCHOR RODS, UNLESS NOTED.
- S8. SIMPLY SUPPORTED BEAM-TO-BEAM CONNECTIONS SHALL BE DOUBLE ANGLE TYPE IN CONFORMANCE WITH THE AISC "MANUAL OF STEEL CONSTRUCTION", UNLESS SPECIFICALLY NOTED OTHERWISE ON THE STRUCTURAL DRAWINGS.
- S9. PROVIDE A 1/4" THICK LEVELING PLATE UNDER EACH COLUMN BASE PLATE FOR USE IN ALIGNING ANCHOR RODS AND BASE PLATES. LEVELING PLATE SHALL BE SET AND GROUTED WITH AN APPROVED NON-SHINK, NON-METALLIC GROUT. GROUT SHALL HAVE ATTAINED DESIGN STRENGTH BEFORE ERECTION OF COLUMN.
- S10. SPLICING STRUCTURAL MEMBERS WHERE NOT DETAILED ON DRAWINGS IS PROHIBITED WITHOUT PRIOR APPROVAL OF ARCHITECT.
- S11. STRUCTURAL STEEL EXPOSED TO THE WEATHER IN THE FINISHED PROJECT SHALL BE HOT DIP GALVANIZED TO CONFORM TO ASTM A123. CANOPY STEEL SHALL BE PRIMED AND PAINTED.
- S12. STRUCTURAL STEEL EXPOSED TO VIEW IN THE COMPLETED PROJECT SHALL BE ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (A.E.S.S.). ALL COLUMNS AND BRACES ON GRID LINES H AND K SHALL BE AESS STEEL. ALL STEEL IN THE CANOPY SHALL BE AESS STEEL. REFER TO SPECIFICATIONS.
- S13. REFER TO THE SPECIFICATION FOR PAINTING AND SURFACE PREPARATION REQUIREMENTS.
- S14. THE CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY GUYING AND BRACING REQUIRED TO ERECT AND HOLD THE NEW STRUCTURE FOR WIND AND CONSTRUCTION LOADS. TEMPORARY SUPPORTS SHALL REMAIN IN PLACE UNTIL ALL ELEMENTS REQUIRED FOR STABILITY OF THE STEEL FRAME ARE COMPLETED.

Ref: All Snips from Atrium Medical Project Documents

Ref: Atrium Medical Project Documents

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Depth Analysis 3:

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

APPENDIX 1

Here are sketches showing what they look like along with dimensions to allow proper clearances when detailing in tight corners... (Exact dimensions should be checked with actual manufacturer's and/or erector technical data)

The Erection Wrenches

This "Connector" tool is used to guide pieces and align holes, hold parts in alignment while bolting, also known as "Spud Wrench" or "Spanner" (works best with a minimum of two holes connection)

The Bull Pins

Are used to "Pull pieces together by hammering its tapered shaft into misaligned holes.

The Drift Pins

Are used to align large connection parts together. It is hammered in and has the same constant diameter as the holes in the connection.

The Torque Guns

Are used to torque bolts to proper tension. Two types are seen on jobs the impact guns (compressed air driven) or the electric guns (used with T.C. bolts). Note that electric guns has a fixed drive and has to be operated in line with bolts.

The Hands

This most important "Connector's" equipment is used for holding the tools, inserting bolts, maneuvering pieces into place, signaling to others... Good detailing practices should always allow enough space to insert that tool for "Making" the connection. Bear in mind that in cold weather it is gloved and needs more space.

TITLE: **The Tools of the Trade**

ERECTOR/FABRICATOR NAME	DRAWN BY	NATIONAL INSTITUTE OF STEEL ERECTORS
JOB	DATE	
JOB NAME	JOB No.	NATIONAL INSTITUTE OF STEEL ERECTORS
	SKETCH No. A1	

Ref: www.NISD.org

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

TITLE: **Beam To Column Web Moment Connection (Suggested)**

ERECTOR/FABRICATOR NAME	DRAWN BY	NATIONAL INSTITUTE OF STEEL ERECTORS
JOB	DATE	
JOB NAME	JOB No.	NATIONAL INSTITUTE OF STEEL ERECTORS
	SKETCH No. S2%	

Ref: www.NISD.org

NISD Industry Standard Details

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

TITLE: **Bolt Access Problems at Small Columns**

ERECTOR/FABRICATOR NAME	DRAWN BY	NATIONAL INSTITUTE OF STEEL ERECTORS
JOB	DATE	
JOB NAME	JOB No.	NATIONAL INSTITUTE OF STEEL ERECTORS
	SKETCH No. S2%	

Ref: www.NISD.org

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

Base Plate Plan View **Base Plate Plan View (With gussets)**

NOTES:

- 1) All columns shall be anchored with a minimum of (4) anchor rods as sized by the design engineer. Each column assembly shall be designed to resist a 300 pound eccentric load located 18" from the column face in any direction at the top of the column.
- 2) (4) rod anchorage alleviates the need for temporary bracing just to hold the column in place, thus is safer and eliminates the chance of the column rolling over on the anchor rods before it can be secured.

New Suggested Sizes for Oversized Holes in Base Plates

Bolt Diameter	Hole Diameter	Bolt Diameter	Hole Diameter
3/4	1 3/16	1 1/2	2 5/16
7/8	1 9/16	1 3/4	2 3/4
1	1 13/16	2	3 1/4
1 1/4	2 1/16	2 1/2	3 3/4

AISC "Manual of Steel Construction", 9th ed., pp. 4-130 lists suggestions for oversized holes for anchor bolts. Based on the trend toward foundation inaccuracy, these allowances are very often not enough. It is suggested that an additional quarter inch over the hole diameter listed be used. A heavy plate washer should be used over the holes (5/16 to 1/2 in. thick). Also refer to the Steel Design Guide Series from AISC "Column Base Plate". Pub. #DB01 Also verify with Design Professionals.

TITLE: **4-Bolts Column Anchorage** 1926-755(a)(1)(2) (OSHA Mandate)

ERECTOR/FABRICATOR NAME	DRAWN BY	NATIONAL INSTITUTE OF STEEL ERECTORS
JOB	DATE	
JOB NAME	JOB No.	NATIONAL INSTITUTE OF STEEL ERECTORS
	SKETCH No. M4	

Ref: www.NISD.org

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Depth Analysis 3:

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

Problem: This typical girt wall detail has a potential for accidents where field or shop workers could hit girts on the sharp edges before the corners are closed in.

Solution: Could be to run one girt past the other to offer a square end. (see also sketch S5)

TITLE: Puncture / Snagging Hazards

ERECTOR/FABRICATOR NAME	DRAWN BY	DATE
JOB	JOB No.	REV. DATE
JOB NAME	SKETCH No. S4	DATE

Logo: NATIONAL INSTITUTE OF STEEL ERECTORS (NISD)

Ref: www.NISD.org

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

BEAM MARKING SYSTEM

Member is swung into place matching marked end with erection mark on plans.

TITLE: Direction North / Safety Connection / Beam Marking (Suggestions)

ERECTOR/FABRICATOR NAME	DRAWN BY	DATE
JOB	JOB No.	REV. DATE
JOB NAME	SKETCH No. A3	DATE

Logo: NATIONAL INSTITUTE OF STEEL ERECTORS (NISD)

Ref: www.NISD.org

NISD Industry Standard Details

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

Solution: Could be to cut out a flange section to allow access

Problem: - This very common situation creates a potentially difficult and dangerous trap.
- Access to bolts holes is not possible for erection wrenches and for torque guns and hands can be caught between beams and wall if not enough space is available.

TITLE: Access Problem / Hand Trap

ERECTOR/FABRICATOR NAME	DRAWN BY	DATE
JOB	JOB No.	REV. DATE
JOB NAME	SKETCH No. S3	DATE

Logo: NATIONAL INSTITUTE OF STEEL ERECTORS (NISD)

Ref: www.NISD.org

DETAILING GUIDE FOR THE ENHANCEMENT OF ERECTION SAFETY

TITLE: The Erector Friendly Column (Suggested)

ERECTOR/FABRICATOR NAME	DRAWN BY	DATE
JOB	JOB No.	REV. DATE
JOB NAME	SKETCH No. S1	DATE

Logo: NATIONAL INSTITUTE OF STEEL ERECTORS (NISD)

Ref: www.NISD.org

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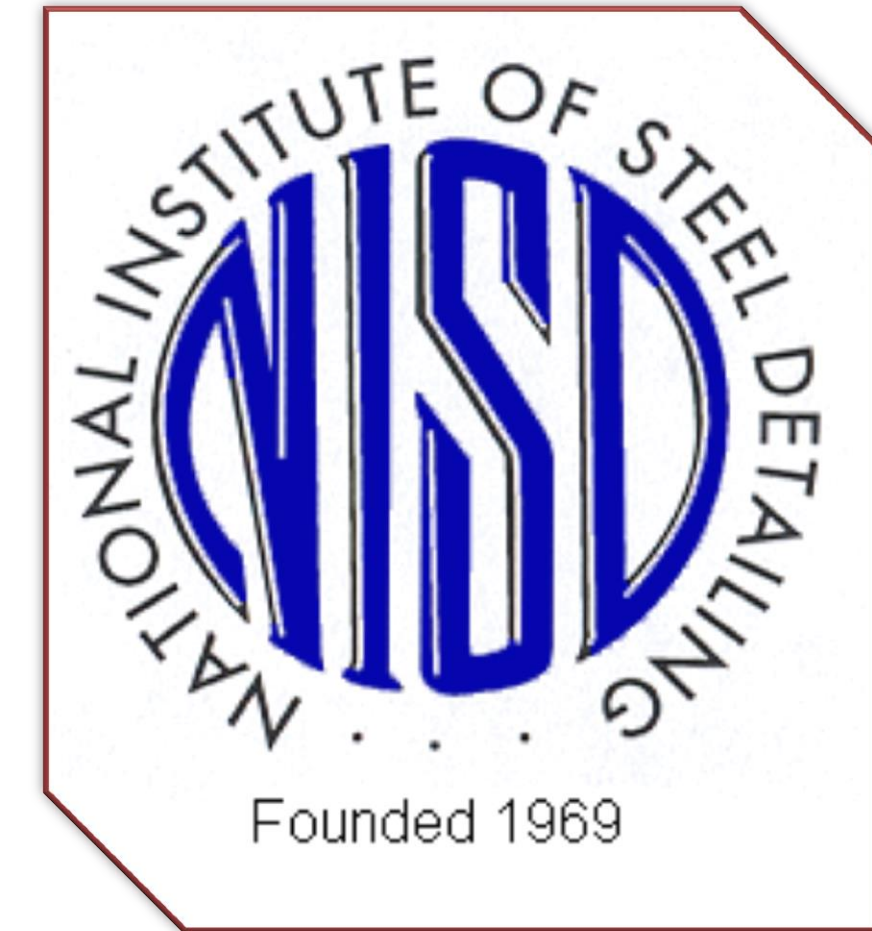


Depth Analysis 3:

Overall Benefits of a Design Guide

- Developed to ensure safety of workers during construction
- If properly implemented, problems can be foreseen and therefore prevented
- Encourages collaboration between designer and constructor
 - Creates a better working relationship, less “lost in translation” incidents
 - Ensures quality control, as issues during design can be managed and adjusted if necessary

Analysis Results



Ref: www.NISD.org

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→ **Conclusions & Recommendations**



Conclusions & Recommendations

Depth Analysis 1:

Conclusion

- Precast structural system imposed a \$1,564,053.00 cost, with the use of (2) 100 ton crawler cranes and 20 to 26.5 days for installation
 - Cost is \$290,893.00 greater than original steel structure
 - 18.5 to 25 days less than structural steel installation

Recommendation

- Install the precast concrete structure, to save time on the critical path of the project schedule

Depth Analysis 2:

Conclusion

- Precast insulated panels cost a total of \$444,219.00, and require 12 to 15 days for installation. The panels also have a thermal efficiency (R-Value) of 23.78
 - Cost is \$74,471.00 greater than original envelope system
 - Installation time is 35 to 38 days less than original envelope
 - R-Value of this system is 1.64 greater than original envelope

Recommendation

- Install the precast insulated panels to save time during installation

Depth Analysis 3:

Conclusion

- Design guide focused on basic steel installation/connection issues, as well as specific details pertaining to connections typically found within Atrium Medical

Recommendation

- Pay the additional upfront fee to hire design professionals and implement a design guide

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All of my family and friends

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Thank you:



David Lage
Les Somero



Bill Moyer
Daniel Zartman



Sean Landry



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Questions?